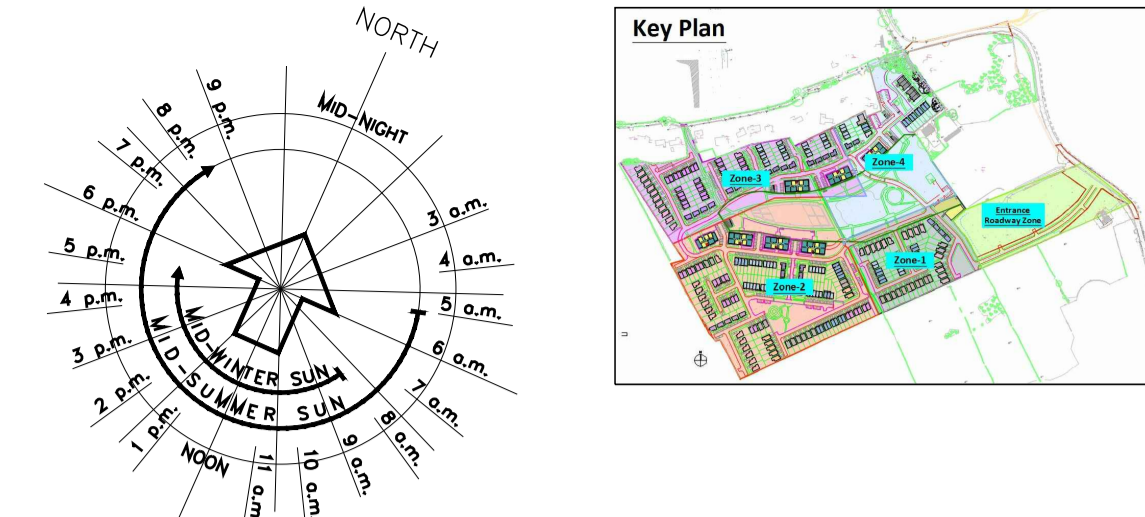


MANHOLE NO.	MANHOLE TYPE	COVER LEVEL (m)	INVERT LEVEL (m)	DEPTH TO SOFFIT (m)	EASTING (m)	NORTHING (m)	
SMH01	1200	Type E	21.500	21.045	1.199	70866.070	80400.880
SMH02	1200	Type D	21.500	20.256	0.796	70866.081	80411.845
SMH03	1200	Type E	20.997	19.815	0.777	70862.781	80423.054
SMH04	1200	Type E	21.500	19.916	1.159	70866.642	80416.954
SMH05	1300	Type E	21.015	19.276	0.993	70862.249	80446.200
SMH06	1200	Type E	20.997	19.098	1.193	70870.072	80417.427
SMH07	1300	Type E	20.527	18.833	1.224	70869.910	80418.413
SMH08	1300	Type E	18.862	17.373	1.309	70846.791	80417.407
SMH09	1200	Type E	21.500	20.227	1.178	70870.142	80408.299
SMH10	1200	Type E	21.511	19.728	1.188	70870.008	80419.810
SMH11	1200	Type B	20.983	19.284	0.846	70873.119	80417.790
SMH12	1200	Type E	18.814	17.133	1.459	70879.880	80418.881
SMH13	1200	Type B	18.862	16.786	1.422	70868.348	80414.585
SMH14	1200	Type E	15.500	13.699	1.279	70867.071	80418.411
SMH15	1200	Type E	19.989	17.708	1.139	70862.369	80418.999
SMH16	1200	Type E	17.862	16.228	1.521	70861.841	80420.980
SMH17	1200	Type B	21.511	18.978	0.608	70861.214	80418.387
SMH18	1200	Type A	20.180	17.798	4.164	70865.021	80420.887
SMH19	1300	Type A	18.641	16.467	3.874	70864.505	80421.748
SMH20	1200	Type E	18.862	16.819	1.247	70869.050	80421.079
SMH21	1200	Type B	18.837	16.218	1.194	70861.157	80421.157
SMH22	1200	Type B	19.989	16.227	3.242	70864.863	80429.799
SMH23	1200	Type E	18.864	17.398	1.191	70862.917	80418.247
SMH24	1300	Type B	17.422	14.877	1.722	70849.162	80427.200
SMH25	1200	Type B	15.525	13.461	1.402	70861.979	80424.268
SMH26	1200	Type D	14.866	13.884	0.887	70864.587	80419.354
SMH27	1200	Type B	13.817	12.689	0.988	70867.627	80423.814
SMH28	1200	Type C	14.375	11.897	1.250	70862.332	80425.707
SMH29	1200	Type E	13.822	12.244	1.133	70861.117	80428.077
SMH30	1200	Type B	14.017	11.799	1.552	70862.919	80427.215
SMH31	1200	Type D	15.217	13.074	0.978	70866.052	80419.418
SMH32	1200	Type E	14.364	13.195	1.029	70869.094	80428.995
SMH33	1200	Type E	14.878	13.131	1.278	70863.339	80421.793
SMH34	1200	Type B	13.383	11.955	1.299	708616.502	80429.582
SMH35	1200	Type B	14.289	12.288	1.784	70192.858	80418.192
SMH36	1200	Type B	13.203	10.897	2.112	70192.218	80424.262
SMH37	1200	Type B	11.871	9.711	1.198	70192.566	80429.802
SMH38	1200	Type E	10.366	8.113	1.398	70194.860	80434.343
SMH39	1200	Type E	10.451	6.928	1.025	70191.162	80434.338
SMH40	1200	Type D	8.239	6.119	0.999	70191.157	80430.414
SMH41	1200	Type D	6.945	4.787	1.741	70867.888	80438.871
SMH42	1200	Type C	9.255	7.785	0.955	70863.558	80435.032
SMH43	HEADWALL	5.840	4.416	1.074	70195.888	80437.889	
SMH44	1200	Type E	15.847	14.241	1.185	70195.514	80440.778



**NOTES:**

THE GREENFIELD RUNOFF RATE OF (QBAR) 105 LITRES/SEC IS CALCULATED ON A SITE AREA OF 17500SQM OR 17.55HA. THE QBAR RATE USED FOR THE CALCULATION OF THE REQUIRED ON SITE STORAGE IS REDUCED TO 80 LITRES/SEC TO COMPENSATE FOR THE EFFECTS OF RUNOFF THAT IS NOT ATTRIBUTED AND WHICH IS DERIVED FROM THE NORTHEAST CORNER OF THE DEVELOPMENT. THE ATTENUATION SYSTEM CONSISTS OF A SILT TRAP, CLASS 3 BYPASS PETROLIUM INTERCEPTION ON ALL NETWORKS AND AN ATTENUATION BASIN/POUND HAVING A STORAGE VOLUME OF CIRCA 25000L. THE CRITICAL STORM DURATION FOR A 1 IN 30 YR STORM EVENT IS 30MIN (WINTER) WHERE THE REQUIRED VOLUME IS 2243.70M3. THE CRITICAL STORM DURATION FOR A 1 IN 100 YR STORM EVENT IS ALSO 30MIN (WINTER) WHERE THE REQUIRED STORAGE VOLUME IS 2978.70M3 RESULTING IN AN OVERFLOW VOLUME OF 115.3M3.

ALL STORM DRAINAGE PIPE LINES HAVE BEEN DESIGNED FOR A 1 IN 2YR RETURN PERIOD WITH A MAXIMUM RAINFALL OF 50MM/H. MINIMUM SELF CLEANING VELOCITY OF 0.8M/SEC AND MINIMUM TIME OF ENTRY 4 MINS. 10% ALLOWANCE HAS BEEN INCLUDED FOR GLOBAL CLIMATE CHANGE.

ALL LEVELS FOR PIPES TO BE CHECKED AND VERIFIED PRIOR TO WORK COMMENCING ON SITE.

THE LAYOUT OF THE BRANCH DRAINS FROM THE INDIVIDUAL SITES ARE AS SHOWN ON THE DWELLINGS LAYOUT PLAN. ANY CHANGES ARE TO BE AGREED PRIOR TO CONSTRUCTION. THE DISTANCE FROM THE FINAL ACCESS JUNCTION ON EACH INDIVIDUAL SITE TO THE CONNECTION TO THE MAIN DRAIN TO BE A MAXIMUM OF 12m.

THE CONNECTION OF THE BRANCH DRAINS TO MAIN DRAINS SHOULD BE MADE AT A MANHOLE WHERE POSSIBLE OR BY USING AN OBLIQUE TYPE SADDLE. SADDLES SHOULD NOT BE USED ON PIPES OF 100MM DIAMETER. NO JOINTS TO CONNECT PIPES OF THE SAME DIAMETER.

ALL PIPES SHOULD HAVE FLEXIBLE JOINTS FORMED BY A METHOD RECOMMENDED BY THE PIPE MANUFACTURER. ELASTOMERIC SEALING RINGS, COMPLYING WITH THE REQUIREMENTS OF BS 2494, TYPE D, SHOULD BE USED.

MANHOLE COVERS AND FRAMES (TO COMPLY WITH THE REQUIREMENTS OF IS EN 24):  
 CLASS 4 FOOTWAYS, GRASS VERGES  
 CLASS 5 ROADWAYS, HARD SHOULDERS, VEHICULAR ACCESSES  
 CLASS 6 AREAS INACCESSIBLE TO MOTOR VEHICLES

ALL BRANCH CONNECTIONS FROM ACCESS JUNCTIONS (AJS) TO BE 100mm uPVC PIPES AT A GRADIENT OF 1 IN 80.

GULLIES SHALL BE PRECAST CONCRETE COMPLYING WITH THE REQUIREMENTS OF BS 8911 PART 200, OR MAY CONSIST OF A CHAMBER CONSTRUCTED OF 100MM SOLID BLOCKWORK AND HAVING A 150MM IN SITU CONCRETE FLOOR, WITH INTERNAL DIMENSIONS OF 450mm x 300mm x 750mm. THE OUTLET FROM THE GULLY SHOULD BE 50MM DIAMETER. SET A MINIMUM OF 375MM ABOVE THE FLOOR OF THE CHAMBER.

GULLY GRADINGS IN ROADS SHOULD BE SET WITH THE DIRECTION OF THE OPENINGS AT RIGHT ANGLES TO THE DIRECTION OF TRAFFIC.

LOCATION AND INVERT LEVELS OF EXISTING (OR PROPOSED) MANHOLES OR OUTFALL POINTS TO BE VERIFIED PRIOR TO COMMENCEMENT OF PROPOSED DRAINAGE NETWORK.

THE TYPE OF PIPE AND FITTINGS TO BE USED TO BE uPVC FOR PIPES UP TO 300mm IN DIAMETER (IN ACCORDANCE WITH THE REQUIREMENTS OF IS EN 240).

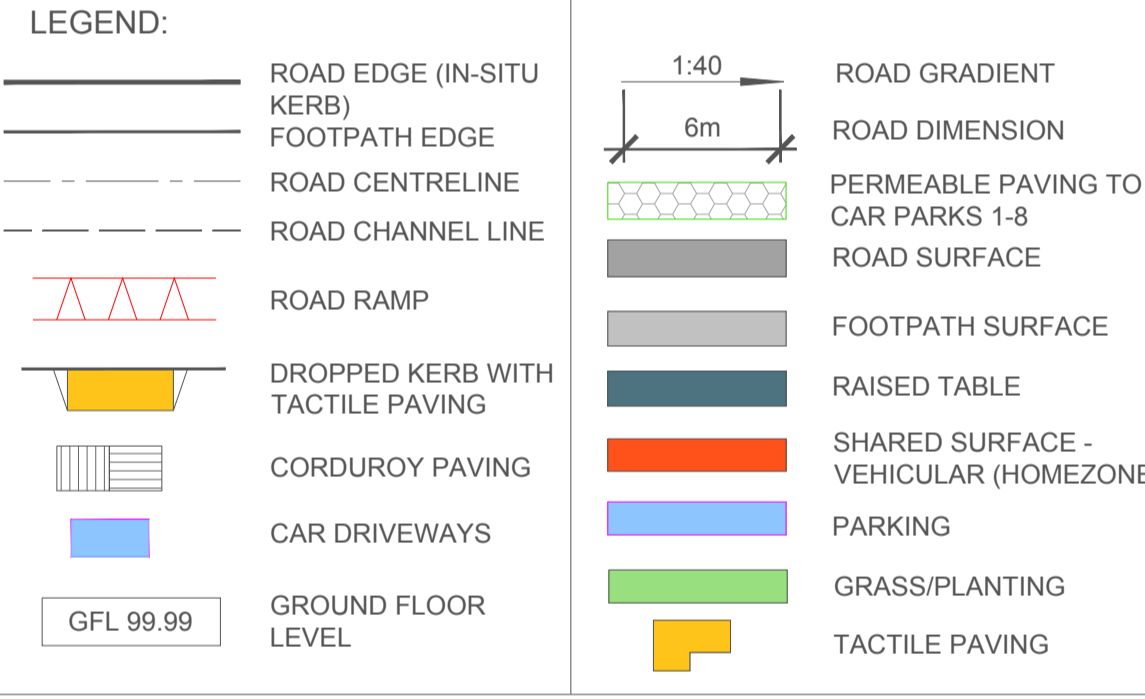
TRENCH WIDTH AT THE LEVEL OF THE TOP OF THE PIPE SHOULD GENERALLY BE AS NARROW AS SAFE WORKING CONDITIONS WOULD ALLOW, WITH A MINIMUM WIDTH OF 300mm PLUS THE EXTERNAL DIAMETER OF THE PIPE BARREL.

DRAINS SHALL BE ACCESSIBLE FOR MAINTENANCE AND REPAIR AND SHALL BE CONSTRUCTED ON PUBLIC PROPERTY. ACCESS SHALL GENERALLY BE PROVIDED BY MEANS OF A MANHOLE BUT SUBJECT TO APPROVAL A PROPRIETARY ACCESS JUNCTION MAY BE USED IN LIEU OF A MANHOLE, ON A DRAIN WHERE THE DEPTH TO INVERT IS LESS THAN 600mm.

FRENCH WIDTH AT THE LEVEL OF THE TOP OF THE PIPE SHOULD GENERALLY BE AS NARROW AS SAFE WORKING CONDITIONS WOULD ALLOW, WITH A MINIMUM WIDTH OF 300mm PLUS THE EXTERNAL DIAMETER OF THE PIPE BARREL.

DRAINS SHALL BE ACCESSIBLE FOR MAINTENANCE AND REPAIR AND SHALL BE CONSTRUCTED ON PUBLIC PROPERTY. ACCESS SHALL GENERALLY BE PROVIDED BY MEANS OF A MANHOLE BUT SUBJECT TO APPROVAL A PROPRIETARY ACCESS JUNCTION MAY BE USED IN LIEU OF A MANHOLE, ON A DRAIN WHERE THE DEPTH TO INVERT IS LESS THAN 600mm.

DRAINAGE PIPES SHOULD BE LAID WITH A MINIMUM COVER OF 1.2m IN ROADS AND DRIVEWAYS, 0.9m IN OPEN SPACES AND FOOTPATHS NOT ADJACENT TO ROADWAYS AND 0.6m IN GARDENS, WHERE IT IS NOT POSSIBLE TO ACHIEVE THESE MINIMUM COVERS, ADDITIONAL MEASURES SHOULD BE TAKEN IN ORDER TO PROTECT FOOTWEAR. DETAILS SHOULD BE AGREED WITH THE ENGINEER PRIOR TO CONSTRUCTING THE PIPELINE.



REV. NO.	DESCRIPTION	DATE	INITIALS
A	Issued for Planning	May 2019	T.Finn

Blakestown, Ardee, Co. Louth, Ireland  
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DRAWING NO: **112A** REV. NO:

TITLE: **Storm Drainage Layout Zone 2**

PROJECT: Residential Development @ Haggardstown, Blackrock, Dundalk

CLIENT: Kingsbridge Consultancy Ltd  
 1st Floor Quayside Business Park  
 Mill Street, Dundalk, Co. Louth.

SCALE: 1:500 @ A1 DRAWN: P.Coyle

DATE: August 2018 CHECKED:

STATUS: **Planning Permission**

JOB NO: **1703**

NOTES:  
 1. Copyright Reserved 2003 ©  
 2. Work to agreed dimensions only. Do not scale drawing.  
 3. The contractor is responsible for checking all levels and dimensions on site and shall refer all discrepancies to the Architect.  
 4. Where appropriate, for details of a/c, structure, or mechanical and electrical details, see Engineers drawings.  
 5. Proprietary items shall be fixed to site in accordance with manufacturers instructions.  
 6. Use of proprietary items shall be checked with manufacturer.  
 7. The contractor shall be responsible for the coordination of structure, finishes and services.

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